

UNIVERSITI TUN HUSSEIN ONN MALAYSIA

FINAL EXAMINATION SEMESTER I SESSION 2015/2016

COURSE NAME

REINFORCED CONCRETE

DESIGN II

COURSE CODE

BFC 32803

PROGRAMME

BACHELOR OF CIVIL

ENGINEERING WITH HONOURS

EXAMINATION DATE :

DECEMBER 2015/JANUARY 2015

DURATION

3 HOURS

INSTRUCTION

1. ANSWER **FOUR (4)** QUESTIONS

ONLY

2. DESIGN SHOULD BE BASED ON:

BS EN 1990:2002+A1:2005

BS EN 1991-1-1:2002 BS EN 1992-1-1:2004

THIS QUESTION PAPER CONSISTS OF ELEVEN (11) PAGES

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A straight longitudinal reinforced concrete stairs supported by reinforced concrete beams at both ends is shown in <u>FIGURE Q1</u>. The staircase is specifically for the public building. Landing slabs at both ends of the stairs are monolithically connected to the stairs. The flight of staircase consists of 10 steps. The average thicknesses of staircase are 189 mm and 279 mm. Given the following data:

Design action of landing $= 12.89 \text{ kN/m}^2$ Design action of flight $= 17.04 \text{ kN/m}^2$ Characteristic strength of concrete $= 25 \text{ N/mm}^2$ Characteristic strength of steel $= 500 \text{ N/mm}^2$ Unit weight of reinforced concrete $= 25 \text{ kN/m}^3$ Nominal concrete cover = 25 mm

Beam size $= 220 \text{ mm} \times 400 \text{ mm}$ Upper Landing area $= 1200 \text{ mm} \times 1000 \text{ mm}$ Lower Landing area $= 1500 \text{ mm} \times 1000 \text{ mm}$

Diameter of reinforcement = 16 mm

(a) The design concept of longitudinal span staircase mainly depends on the condition between flight and landing. If the flight and landing should be designed separately, describe the characteristics of the staircase. Give one example of the staircase.

(4 marks)

(b) Determine the suitable size of riser, going and waist.

(5 marks)

(c) Determine the total action, bending moment and shear force.

(5 marks)

(d) Design shear resistance of the staircase if required longitudinal reinforcement is only 20% of maximum main reinforcement.

(11 marks)

Q2 (a) Name and explain briefly, TWO (2) levels of sub-frame analysis.

(4 marks)

(b) <u>FIGURE 2(a)</u> shows a four-storey braced frame while <u>FIGURE 2(b)</u> shows the bending moment diagram for column A from first to third floor. By using the given data, calculate moment of inertia and stiffness for beam and column in <u>FIGURE 2(b)</u>.

Size of all columns: 300 x 300 mm Size of all beams: 150 x 300 mm

(5 marks)

(c) Determine the design load, w (in unit kN/m) for beam 2/A-B by using value of upper and lower column moment in **FIGURE 2(b)**. Consider simplified sub-frame at a point analysis.

(5 marks)

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(d) Analyze Beam 2/A-B using the design load of 30 kN/m and 25 kN/m for span A-B and B-C respectively. Present your end results by drawing the shear force and bending moment diagram for Beam 2/A-B. The distribution factor is given as follows:

Joint A:

$$F_{AB} = 0.12$$

$$F_{cu}=0.47$$

Joint B:

$$F_{cl} = 0.41$$

 $F_{BA} = 0.12$

$$F_{BC} = 0.06$$

 $F_{cu} = 0.44$
 $F_{cl} = 0.38$

(11 marks)

Q3 (a) Define a slender column in the reinforced concrete building.

(1 mark)

(b) Explain how to design a short column with TWO (2) bending moments.

(3 marks)

- FIGURE Q3 shows a braced frame structure of a building. (c)
 - (i) Classify the column B either short or slender. The connection between column and footing is assumed to be fixed. The calculation of effective height, l_o should be based on BS 8110 approach. $N_{Ed} = 320$ kN, $f_{ck} = 30$ N/mm^2 .

(5 marks)

(ii) Calculate the M_{Ed} for column A and B. The loading condition must be based on critical loading. Use l_o as in (i)

(5 marks)

(iii) From (ii), design the main reinforcement and the link for column A and B. $(f_{vk} = 500 \text{ N/mm}^2, d_2 / h = 0.15)$

(11 marks)

Q4 List FOUR (4) steps that need to be considered in pile design. (a)

(4 marks)

(b) Describe briefly TWO (2) theories in pile cap design.

(5 marks)

(c) A pile foundation needs to be designed for an office building at soft soil area. It is required to support permanent axial action 4000 kN and variable action 3000 kN from a 450 x 450 mm rectangular reinforced concrete column. Service load capacity for the pile is 2000 kN. The diameter of pile is 600 mm.

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(i) Determine the suitable number of pile and size of pile cap. Draw the detail size of pile cap with the exact dimensions.

(11 marks)

(ii) Design the main reinforcement of the pile.

(5 marks)

Q5 (a) Specific the function of retaining wall and describe the compulsory elements that should be designed for a retaining wall to retain well-drained soil with depth more than 5 m.

(4 marks)

(b) A cantilever wall as shown in <u>FIGURE Q5</u> support a bank earth 5.0 m height. The soil behind the wall has density of 1900 kg/m³ with angle of internal friction of 30°. Given the following data:

Characteristic strength of concrete = 35 N/mm^2 Characteristic strength of steel = 500 N/mm^2

Coefficient of friction = 0.5 Soil cohesion = 0

Soil bearing pressure = 150 kN/m^2 Earth pressure = 31.67 kN/m^2

(i) Determine the total horizontal load and moment of the retaining wall.

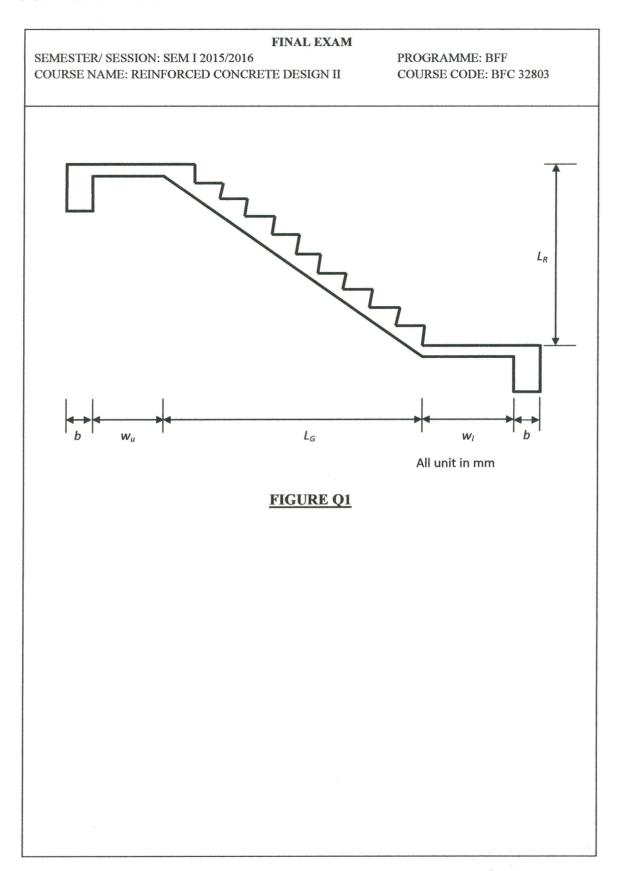
(5 marks)

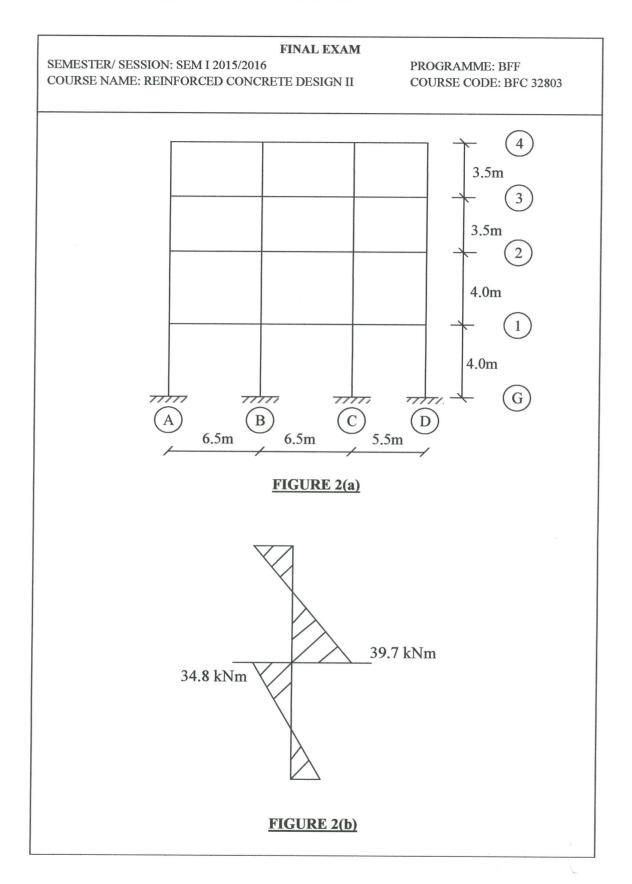
(ii) Check the soil pressure at the toe of retaining wall.

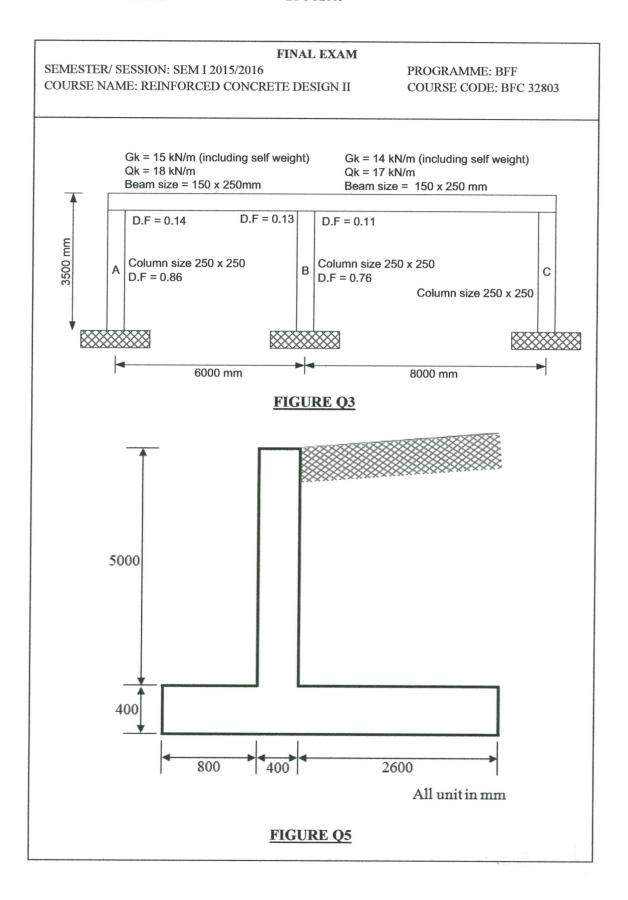
(5 marks)

(iii) Predict the ability of retaining wall toward stability against overturning and resistance to sliding.

(11 marks)







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APPENDIX

Table 1.0: Comfort criteria of staircase

| Туре | Riser | Going | | |
|------------------|---------------------|----------------------------------|--|--|
| Public Building | Not more than 180mm | Not be less than 255mm | | |
| Private Building | Not more than 200mm | Varies between 250mm to 400mm | | |

$$t = h \left\lceil \frac{\left(G^2 + R^2\right)^{1/2}}{G} \right\rceil + \frac{R}{2}$$

$$z = d \left[0.5 + \sqrt{0.25 - \left(\frac{K}{1.134} \right)} \right]$$

$$A_{s} = \frac{M}{0.87 f_{vk} z}$$

$$A_{s,min} = 0.26 \left(\frac{f_{\text{ctm}}}{f_{\text{yk}}} \right) bd$$

$$V_{Rd,c} = \left[0.12k (100 \rho_1 f_{ck})^{1/3} \right] bd$$

$$k = 1 + \left(\frac{200}{d}\right)^{1/2}$$

$$\rho_1 = \frac{A_s}{bd} \le 0.02$$

$$P = \gamma z \left(\frac{1 - \sin \phi}{1 + \sin \phi} \right)$$

$$P_{\text{max}} = \frac{\sum W}{A} \pm \frac{\sum M}{Z}$$

$$P_{1} = 0.5\gamma H_{1}^{2} \left(\frac{1 - \sin \phi}{1 + \sin \phi} \right)$$
 $P_{2} = 0.5\gamma H_{2}^{2} \left(\frac{1 + \sin \phi}{1 - \sin \phi} \right)$

$$P_2 = 0.5\gamma H_2^2 \left(\frac{1 + \sin \phi}{1 - \sin \phi} \right)$$

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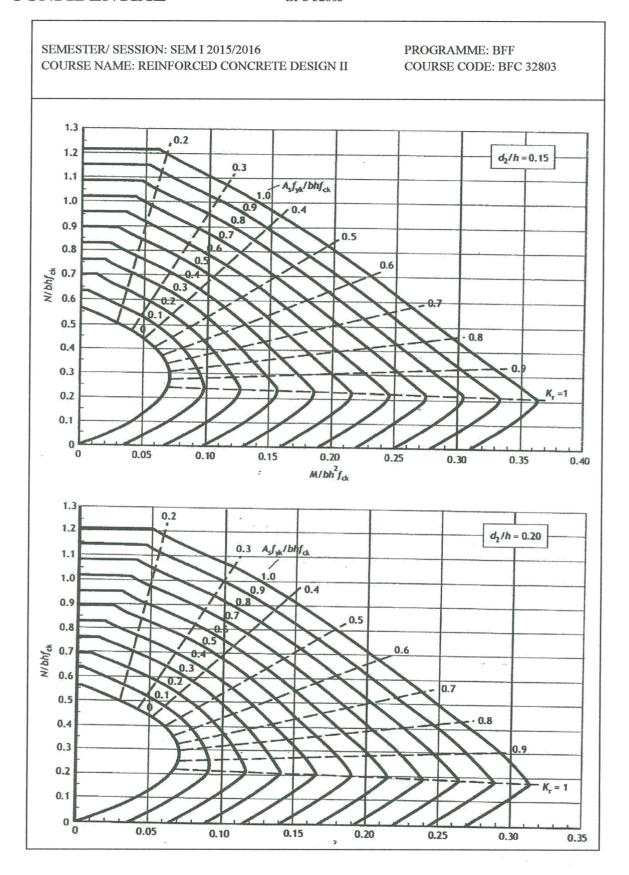
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Table 2.0: Cross Sectional Area (mm²) according to Size and Numbers of Bar

| Bar Size | Number of Bar | | | | | | | Perimeter (mm) | |
|-------------|---------------|------|------|------|------|------|------|----------------|-------|
| (mm) | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | - |
| 6 | 28.3 | 56.6 | 84.9 | 113 | 141 | 170 | 198 | 226 | 18.9 |
| 8 | 50.3 | 101 | 151 | 201 | 251 | 302 | 352 | 402 | 25.1 |
| 10 | 78.6 | 157 | 236 | 314 | 393 | 471 | 550 | 629 | 31.4 |
| | | | | | | | | | |
| 12 | 113 | 226 | 339 | 453 | 566 | 679 | 792 | 905 | 37.7 |
| 16 | 201 | 402 | 603 | 805 | 1006 | 1207 | 1408 | 1609 | 50.3 |
| 20 | 314 | 629 | 943 | 1257 | 1571 | 1886 | 2200 | 2514 | 62.9 |
| | | | | | | | | | |
| 25 | 491 | 982 | 1473 | 1964 | 2455 | 2946 | 3438 | 3929 | 78.6 |
| 32 | 805 | 1609 | 2414 | 3218 | 4023 | 4827 | 5632 | 6437 | 100.6 |
| 40 | 1257 | 2514 | 3771 | 5029 | 6286 | 7543 | 8800 | 10057 | 125.7 |

Table 3.0: Cross Sectional Area (mm²) for every meter width at distance between Bar

| Bar Size | Distance between Bar (mm) | | | | | | | | |
|-------------|---------------------------|-------|-------|-------|------|------|------|------|------|
| (mm) | 50 | 75 | 100 | 125 | 150 | 175 | 200 | 250 | 300 |
| 6 | 566 | 377 | 283 | 226 | 189 | 162 | 141 | 113 | 94 |
| 8 | 1006 | 670 | 503 | 402 | 335 | 287 | 251 | 201 | 168 |
| 10 | 1571 | 1048 | 786 | 629 | 524 | 449 | 393 | 314 | 262 |
| 12 | 2263 | 1509 | 1131 | 905 | 754 | 647 | 566 | 453 | 377 |
| 16 | 4023 | 2682 | 2011 | 1609 | 1341 | 1149 | 1006 | 805 | 670 |
| 20 | 6286 | 4190 | 3143 | 2514 | 2095 | 1796 | 1571 | 1257 | 1048 |
| 25 | 9821 | 6548 | 4911 | 3929 | 3274 | 2806 | 2455 | 1964 | 1637 |
| 32 | 16091 | 10728 | 8046 | 6437 | 5364 | 4598 | 4023 | 3218 | 2682 |
| 40 | 25143 | 16762 | 12571 | 10057 | 8381 | 7184 | 6286 | 5029 | 4190 |



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| $h = 2h_p + 100$; If $h_p > 550 \text{ mm}$: $h = \frac{1}{3}(8h_p - 600)$ | Tensile force to be resisted by reinforcement ng size Taking size of column umn into consideration | $\frac{N}{121d}(3l^2 - a^2)$ | Parallel to X-X: $\frac{N}{361d}(4l^2 + b^2 - 3a^2)$ | Parallel to Y-Y: $\frac{N}{18ld}(2l^2 - b^2)$ | Parallel to X-X: $\frac{N}{24Id}(3I^2 - a^2)$ | Parallel to Y-Y: $\frac{N}{24ld}(3l^2 - b^2)$ | Parallel to X-X: $\frac{N}{301d}(3l^2 - a^2)$ | Parallel to Y.Y: $\frac{N}{3014}(3l^2 - b^2)$ | spacing factor of piles (normally between 2 and 3 depending on ground conditions) |
|--|--|--|---|--|---|---|---|---|---|
| If hp > 550 mm: h= | Tensile for Neglecting size of column | NI 44 | INI | р6 | NI | P8 | IN | <i>P</i> 01 | of piles (normally betw |
| | Dimensions of pile cap | $\frac{l}{r} = ah_{o}$ $\frac{r}{r} = \frac{1}{r}$ | 000 + °4 (1 + 4) 000 + °4 (1 | $\frac{1}{(\alpha+1)} \frac{1}{h_o} + \frac{1}{300}$ | ************************************** | (a+1)h _p +300 | × × × × × × × × × × × × × × × × × × × | [\sqrt{2}] | a,b dimensions of column; a |
| | Number of piles | 2 | т | and the state of t | * | 1 | · · | S | Notation h, diameter of pile; |
| | | | | | | | 9 | | |