



**UTHM**

Universiti Tun Hussein Onn Malaysia

**UNIVERSITI TUN HUSSEIN ONN MALAYSIA**

**FINAL EXAMINATION  
SEMESTER I  
SESSION 2015/2016**

COURSE NAME : REINFORCED CONCRETE  
DESIGN II

COURSE CODE : BFC 32802

PROGRAMME : BACHELOR OF CIVIL  
ENGINEERING WITH HONOURS

EXAMINATION DATE : DECEMBER 2015/JANUARY 2015

DURATION : 2 HOURS AND 30 MINUTES

INSTRUCTION : 1. ANSWER **FOUR (4)** QUESTIONS  
ONLY

2. DESIGN SHOULD BE BASED ON:  
BS EN 1990:2002+A1:2005  
BS EN 1991-1-1:2002  
BS EN 1992-1-1:2004

THIS QUESTION PAPER CONSISTS OF **ELEVEN (11)** PAGES

- Q1** A straight longitudinal reinforced concrete stairs supported by reinforced concrete beams at both ends is shown in **FIGURE Q1**. The staircase is specifically for the public building. Landing slabs at both ends of the stairs are monolithically connected to the stairs. The flight of staircase consists of 10 steps. The average thicknesses of staircase are 189 mm and 279 mm. Given the following data:

Design action of landing	= 12.89 kN/m <sup>2</sup>
Design action of flight	= 17.04 kN/m <sup>2</sup>
Characteristic strength of concrete	= 25 N/mm <sup>2</sup>
Characteristic strength of steel	= 500 N/mm <sup>2</sup>
Unit weight of reinforced concrete	= 25 kN/m <sup>3</sup>
Nominal concrete cover	= 25 mm
Beam size	= 220 mm x 400 mm
Upper Landing area	= 1200 mm x 1000 mm
Lower Landing area	= 1500 mm x 1000 mm
Diameter of reinforcement	= 16 mm

- (a) Determine the suitable size of riser, going and waist. (7 marks)
- (b) Determine the total action, bending moment and shear force. (7 marks)
- (c) Design shear resistance of the staircase if required longitudinal reinforcement is only 20% of maximum main reinforcement. (11 marks)
- Q2** (a) **FIGURE 2(a)** shows a four-storey braced frame while **FIGURE 2(b)** shows the bending moment diagram for column A from first to third floor. By using the given data, calculate moment of inertia and stiffness for beam and column in **FIGURE 2(b)**.
- Size of all column: 300 x 300 mm  
Size of all beam: 400 x 900 mm (7 marks)
- (b) Determine the design load,  $w$  (in unit kN/m) for beam 2/A-B by using value of upper and lower column moment in **FIGURE 2(b)**. Consider simplified sub-frame at a point analysis. (7 mark)

- (c) Analyze Beam 2/A-B using the design load of 30 kN/m and 25 kN/m for span A-B and B-C respectively. Present your end results by drawing the shear force and bending moment diagram for Beam 2/A-B. The distribution factor is given as follows:

$$\begin{aligned} \text{Joint A:} \quad & F_{AB} = 0.12 \\ & F_{cu} = 0.47 \\ & F_{cl} = 0.41 \\ \text{Joint B:} \quad & F_{BA} = 0.12 \\ & F_{BC} = 0.06 \\ & F_{cu} = 0.44 \\ & F_{cl} = 0.38 \end{aligned}$$

(11 marks)

**Q3** FIGURE Q3 shows a braced frame structure of a building.

- (a) Classify the column B either short or slender. The connection between column and footing is assumed to be fixed. The calculation of effective height,  $l_o$  should be based on BS 8110 approach.  $N_{Ed} = 320 \text{ kN}$ ,  $f_{ck} = 30 \text{ N/mm}^2$ . (7 marks)
- (b) Calculate the  $M_{Ed}$  for column A and B. The loading condition must be based on critical loading. Use effective height,  $l_o$  as in **Q3(a)**. (7 marks)
- (c) From **Q3(b)**, design the main reinforcement and the link for column A and B. ( $f_{yk} = 500 \text{ N/mm}^2$ ,  $d_2/h = 0.15$ ) (11 marks)

**Q4** (a) Describe briefly **TWO (2)** theories in pile cap design. (7 marks)

- (b) A pile foundation needs to be designed for an office building at soft soil area. It is required to support permanent axial action 4000 kN and variable action 3000 kN from a 450 x 450 mm rectangular reinforced concrete column. Service load capacity for the pile is 2000 kN. The diameter of pile is 600 mm.

- (i) Determine the suitable number of pile and size of pile cap. Draw the detail size of pile cap with the exact dimensions. (11 marks)
- (ii) Design the main reinforcement of the pile. (7 marks)

**Q5** A cantilever wall as shown in **FIGURE Q5** support a bank earth 5.0 m height. The soil behind the wall has density of  $1900 \text{ kg/m}^3$  with angle of internal friction of  $30^\circ$ . Given the following data:

Characteristic strength of concrete	= $35 \text{ N/mm}^2$
Characteristic strength of steel	= $500 \text{ N/mm}^2$
Coefficient of friction	= 0.5
Soil cohesion	= 0
Soil bearing pressure	= $150 \text{ kN/m}^2$
Earth pressure	= $31.67 \text{ kN/m}^2$

- (a) Determine the total horizontal load and moment of the retaining wall. (7 marks)
- (b) Check the soil pressure at the toe of retaining wall. (7 marks)
- (c) Predict the ability of retaining wall toward stability against overturning and resistance to sliding. (11 marks)

**- END OF QUESTIONS -**

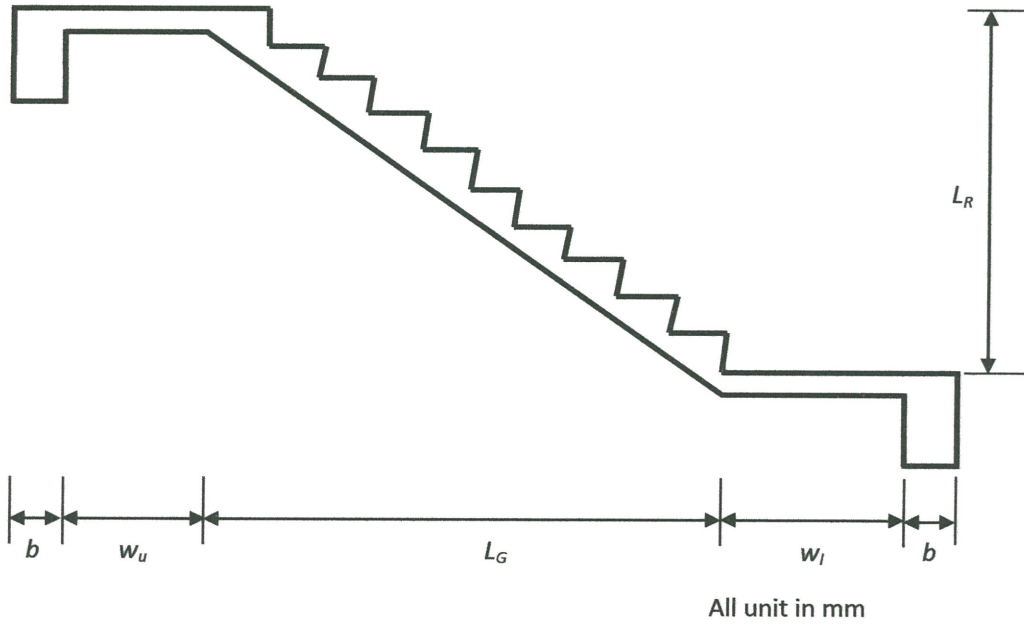
**FINAL EXAM**

SEMESTER/ SESSION: SEM I 2015/2016

PROGRAMME: BFF

COURSE NAME: REINFORCED CONCRETE DESIGN II

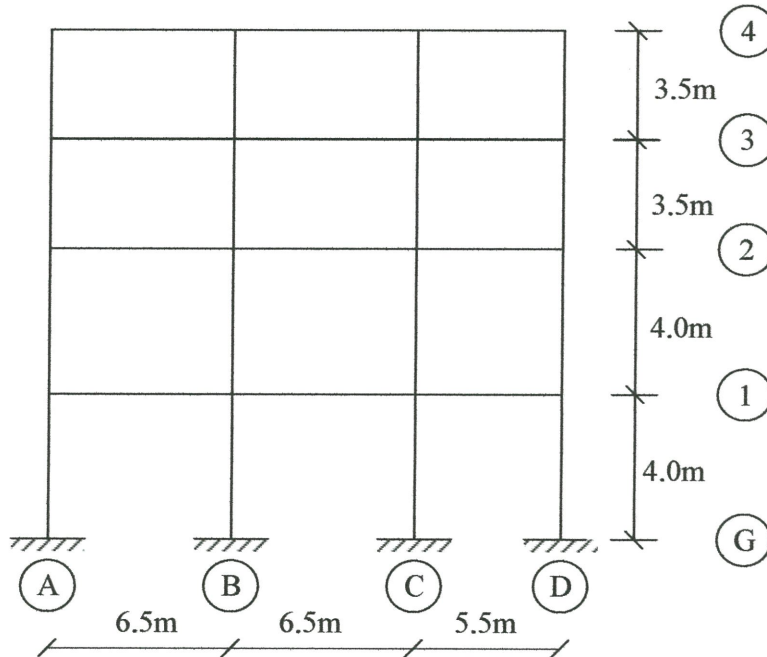
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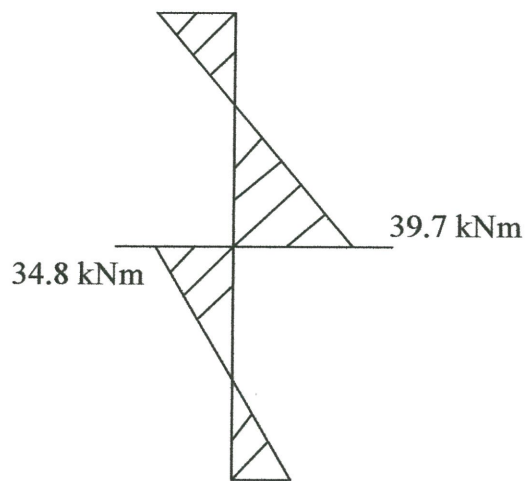
**FIGURE Q1**

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SEMESTER/ SESSION: SEM I 2015/2016 PROGRAMME: BFF  
 COURSE NAME: REINFORCED CONCRETE DESIGN II COURSE CODE: BFC 32802



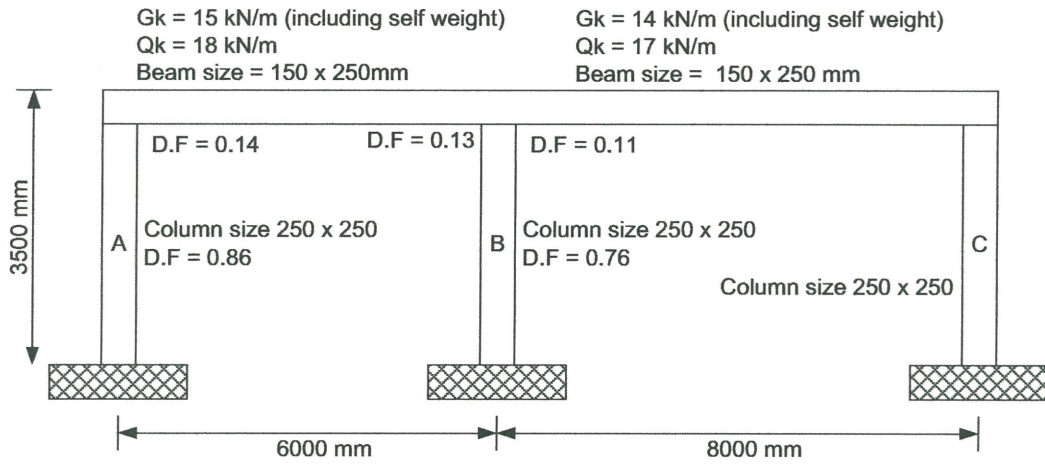
**FIGURE 2(a)**



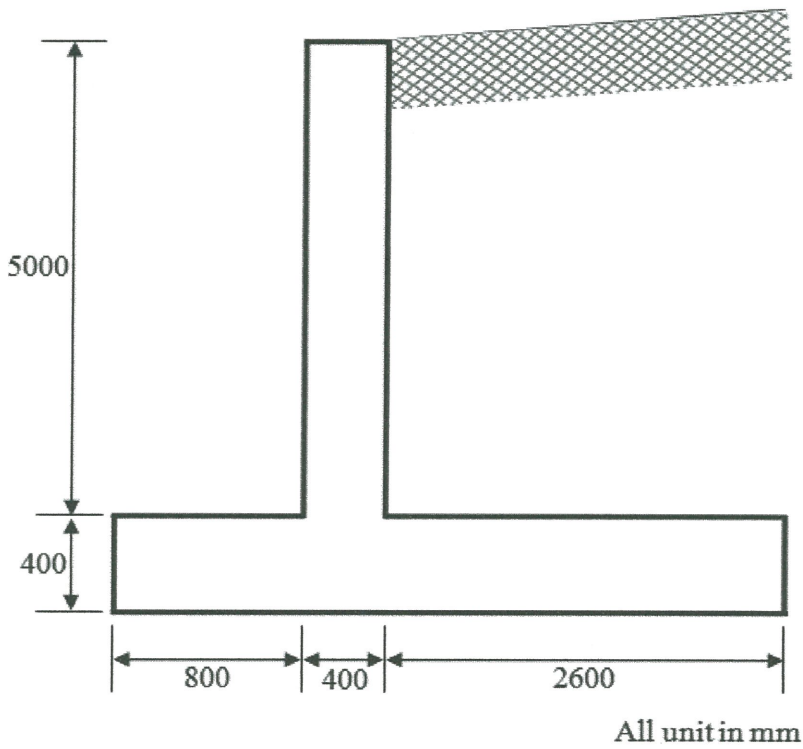
**FIGURE 2(b)**

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SEMESTER/ SESSION: SEM I 2015/2016 PROGRAMME: BFF  
 COURSE NAME: REINFORCED CONCRETE DESIGN II COURSE CODE: BFC 32802



**FIGURE Q3**



All unit in mm

**FIGURE Q5**

**FINAL EXAM**

SEMESTER/ SESSION: SEM I 2015/2016 PROGRAMME: BFF  
 COURSE NAME: REINFORCED CONCRETE DESIGN II COURSE CODE: BFC 32802

**APPENDIX**

**Table 1.0: Comfort criteria of staircase**

Type	Riser	Going
Public Building	Not more than 180mm	Not be less than 255mm
Private Building	Not more than 200mm	Varies between 250mm to 400mm

$$t = h \left[ \frac{(G^2 + R^2)^{1/2}}{G} \right] + \frac{R}{2}$$

$$z = d \left[ 0.5 + \sqrt{0.25 - \left( \frac{K}{1.134} \right)} \right]$$

$$A_s = \frac{M}{0.87 f_{yk} z}$$

$$A_{s,min} = 0.26 \left( \frac{f_{ctm}}{f_{yk}} \right) bd$$

$$V_{Rd,c} = \left[ 0.12k (100 \rho_1 f_{ck})^{1/3} \right] bd$$

$$k = 1 + \left( \frac{200}{d} \right)^{1/2}$$

$$\rho_1 = \frac{A_s}{bd} \leq 0.02$$

$$P = \gamma z \left( \frac{1 - \sin \phi}{1 + \sin \phi} \right)$$

$$P_{max} = \frac{\sum W}{A} \pm \frac{\sum M}{Z}$$

$$P_1 = 0.5 \gamma H_1^2 \left( \frac{1 - \sin \phi}{1 + \sin \phi} \right)$$

$$P_2 = 0.5 \gamma H_2^2 \left( \frac{1 + \sin \phi}{1 - \sin \phi} \right)$$



<b>FINAL EXAM</b>	
SEMESTER/ SESSION: SEM I 2015/2016	PROGRAMME: BFF
COURSE NAME: REINFORCED CONCRETE DESIGN II	COURSE CODE: BFC 32802

**Table 2.0: Cross Sectional Area (mm<sup>2</sup>) according to Size and Numbers of Bar**

Bar Size (mm)	Number of Bar								Perimeter (mm)
	1	2	3	4	5	6	7	8	
6	28.3	56.6	84.9	113	141	170	198	226	18.9
8	50.3	101	151	201	251	302	352	402	25.1
10	78.6	157	236	314	393	471	550	629	31.4
12	113	226	339	453	566	679	792	905	37.7
16	201	402	603	805	1006	1207	1408	1609	50.3
20	314	629	943	1257	1571	1886	2200	2514	62.9
25	491	982	1473	1964	2455	2946	3438	3929	78.6
32	805	1609	2414	3218	4023	4827	5632	6437	100.6
40	1257	2514	3771	5029	6286	7543	8800	10057	125.7

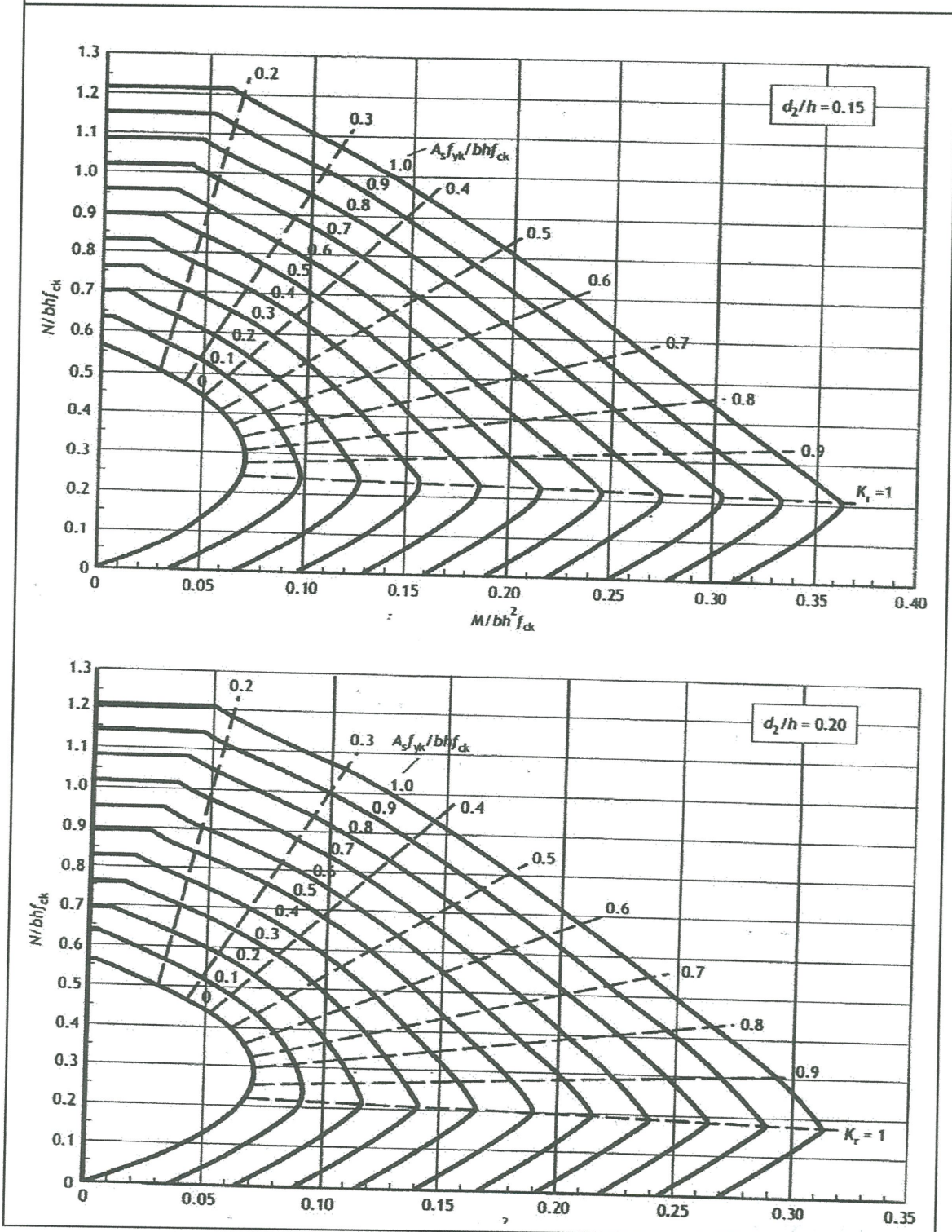
**Table 3.0: Cross Sectional Area (mm<sup>2</sup>) for every meter width at distance between Bar**

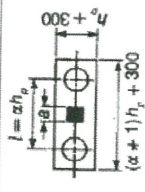
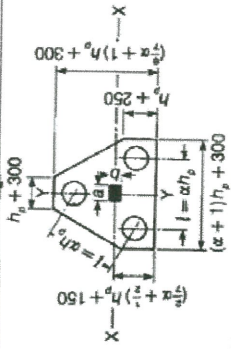
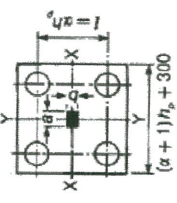
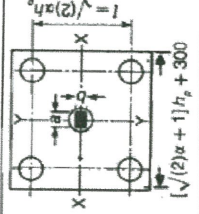
Bar Size (mm)	Distance between Bar (mm)								
	50	75	100	125	150	175	200	250	300
6	566	377	283	226	189	162	141	113	94
8	1006	670	503	402	335	287	251	201	168
10	1571	1048	786	629	524	449	393	314	262
12	2263	1509	1131	905	754	647	566	453	377
16	4023	2682	2011	1609	1341	1149	1006	805	670
20	6286	4190	3143	2514	2095	1796	1571	1257	1048
25	9821	6548	4911	3929	3274	2806	2455	1964	1637
32	16091	10728	8046	6437	5364	4598	4023	3218	2682
40	25143	16762	12571	10057	8381	7184	6286	5029	4190

FINAL EXAM

SEMESTER/ SESSION: SEM I 2015/2016  
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SEMESTER/ SESSION: SEM I 2015/2016		FINAL EXAM		PROGRAMME: BFF	
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If $h_p \leq 550$ mm: $h = 2h_p + 100$ ; If $h_p > 550$ mm: $h = \frac{1}{3}(8h_p - 600)$					
Number of piles	Dimensions of pile cap	Neglecting size of column	Tensile force to be resisted by reinforcement Taking size of column into consideration		
2		$\frac{Nl}{4d}$	$\frac{N}{12ld}(3l^2 - a^2)$		
3		$\frac{Nl}{9d}$	Parallel to X-X: $\frac{N}{36ld}(4l^2 + b^2 - 3a^2)$ Parallel to Y-Y: $\frac{N}{18ld}(2l^2 - b^2)$		
4		$\frac{Nl}{8d}$	Parallel to X-X: $\frac{N}{24ld}(3l^2 - a^2)$ Parallel to Y-Y: $\frac{N}{24ld}(3l^2 - b^2)$		
5		$\frac{Nl}{10d}$	Parallel to X-X: $\frac{N}{30ld}(3l^2 - a^2)$ Parallel to Y-Y: $\frac{N}{30ld}(3l^2 - b^2)$		
<p><b>Notation</b> <math>h_p</math> diameter of pile; <math>a, b</math> dimensions of column; <math>\alpha</math> spacing factor of piles (normally between 2 and 3 depending on ground conditions)</p>					