



**UNIVERSITI TUN HUSSEIN ONN MALAYSIA**

**FINAL EXAMINATION  
SEMESTER I  
SESSION 2015/2016**

COURSE NAME : GEOTECHNICS I  
COURSE CODE : BFC 21702  
PROGRAMME : BACHELOR OF CIVIL  
ENGINEERING WITH HONOURS  
EXAMINATION DATE : DECEMBER 2015 / JANUARY 2016  
DURATION : 2 HOURS  
INSTRUCTION : ANSWER **ALL** QUESTIONS IN  
PART A AND ANY **TWO (2)**  
QUESTIONS FROM PART B

THIS QUESTION PAPER CONSISTS OF **NINE (9)** PAGES

**PART A**

- Q1** (a) One of the important parameters needed in designing a shallow foundation on soil at a given site is its shear strength parameters.
- (i) Explain what shear strength parameters are. (4 marks)
  - (ii) Briefly describe how these parameters are measured in a laboratory using a direct shear test method with the aid of sketches. (6 marks)
  - (iii) Briefly explain how shear strength are measured in-situ using vane shear test method. (6 marks)
- (b) The results of three Consolidated-Undrained triaxial tests on identical soil specimens of a particular soil are listed in **TABLE Q1** below.:

**TABLE Q1**

Test No.	Confining pressure (kN/m <sup>2</sup> )	Deviator stress at failure (kN/m <sup>2</sup> )	Pore pressure at failure (kN/m <sup>2</sup> )
1	200	244	55
2	300	314	107
3	400	384	159

- (i) Determine the total and the effective shear strength parameters of the soil. (10 marks)
- (ii) What would be the expected deviator stress and the pore pressure at failure for a test with the confining pressure of 100 kPa. (6 marks)
- (iii) Comments on the usage of both the total and the effective shear strength parameters of the soil in designing geotechnical engineering structures. (8 marks)

**PART B**

- Q2** (a) Soil in nature is a three-phase system which consists of solid soil particles, and void containing water and air. Water has mass and weight while air is mass-less but it occupies part of the soil volume.

Based on that statement,

- (i) What is a fully saturated soil? (2 marks)
- (ii) Define the porosity of soil. (2 marks)

- (b) Some representative soil specimen has been collected from a field embankment. The specific gravity and moist unit weight of the soil are determined as 2.67 and 17.61 kN/m<sup>3</sup> respectively in laboratory while moisture content is found to be 10.8 %.

Determine;

- (i) Dry unit weight of soil (2 marks)
- (ii) Void ratio of soil (2 marks)
- (iii) Porosity (2 marks)
- (iv) Degree of saturation (2 marks)
- (v) If the degree of saturation of soil is required to be 80%, determine the weight of water (in kN) that need to be added per cubic meter of soil. (4 marks)

- (c) The particle size distribution of soil determined from sieve analysis is shown in **TABLE Q2**.

**TABLE Q2**: Sieve analysis results

Sieve size (mm)	Mass of soil retained on each sieve (g)
4.75	0.0
2.00	18.5
0.850	53.2
0.425	90.5
0.250	81.8
0.150	92.2
0.075	58.5
Pan	26.5

- (i) Determine the percentage finer than each sieve size and plot a grain-size distribution curve. (7 marks)
- (ii) Determine  $D_{10}$ ,  $D_{30}$  and  $D_{60}$  from the grain-size distribution curve. (3 marks)
- (iii) Calculate the uniformity coefficient,  $C_u$ . (2 marks)
- (iv) Calculate the coefficient of gradation,  $C_c$ . (2 marks)

- Q3** (a) “Compaction is the process of packing together the soil particles by reducing the air voids”.

Explain with the aid of appropriate diagrams, how this process is influenced by;

- (i) increasing the moisture content

(3 marks)

- (ii) increasing the compaction energy

(3 marks)

- (b) Laboratory observations from a standard Proctor compaction test carried out on a soil sample collected from a highway construction site is given in the **TABLE Q3** below. The volume and mass of the empty Proctor mould was 943 cm<sup>3</sup> and 2.4 kg respectively.

**TABLE Q3:** Standard Proctor compaction test results

Moisture content (%)	9.9	10.6	12.1	13.8	15.1	17.4	19.4	21.2
Mass of the mould and wet compacted soil (kg)	4.08	4.11	4.17	4.23	4.26	4.28	4.27	4.25

Determine the maximum dry unit weight (kN/m<sup>3</sup>), moisture content and the air porosity of the optimum sample.

(15 marks)

- (c) The earthwork requirements at the highway construction site specify a relative compaction of 92.5%. The field characteristics determined for a compacted soil sample from the construction site is 10.5% and 1700 kg/m<sup>3</sup> moisture content and moist density respectively.

Investigate and comment on the level of compaction at the construction site.

(9 marks)

- Q4** (a) Briefly explain what is referred as boiling or quick condition and how this condition will happen.

(6 marks)

- (b) Effective stress of saturated soil under seepage condition is found to be different from the condition without seepage.

Please explain what is the effect of

- (i) upward seepage  
(ii) downward seepage

on the effective stress of saturated soil.

(6 marks)

- (c) In a falling head permeability test on fine sand, the sample is 75 mm diameter and 150 mm length and the standpipe 10 mm diameter. A stopwatch is started when  $h = 500$  mm and stopped when  $h = 250$  mm and reads 19.6s. The test is repeated for a drop from 250 mm to 125 mm and the time is 19.4 s.

Determine the coefficient of permeability of fine sand in mm/s.

(8 marks)

- (d) A pumping test was carried out to determine the coefficient of permeability of soil site which was selected for the construction of an earth dam. Observation wells were established at a distance of 3 m and 6 m for the test well.

The following data were obtained :

Depth of water table	= 16 m
Discharge under steady condition	= $2.3\text{m}^3/\text{min}$
Drawdown at outer well	= 0.50 m
Drawdown at inner well	= 1.5 m

Determine the coefficient of permeability of the soil in m/min.

(10 marks)

- **END OF QUESTION** -

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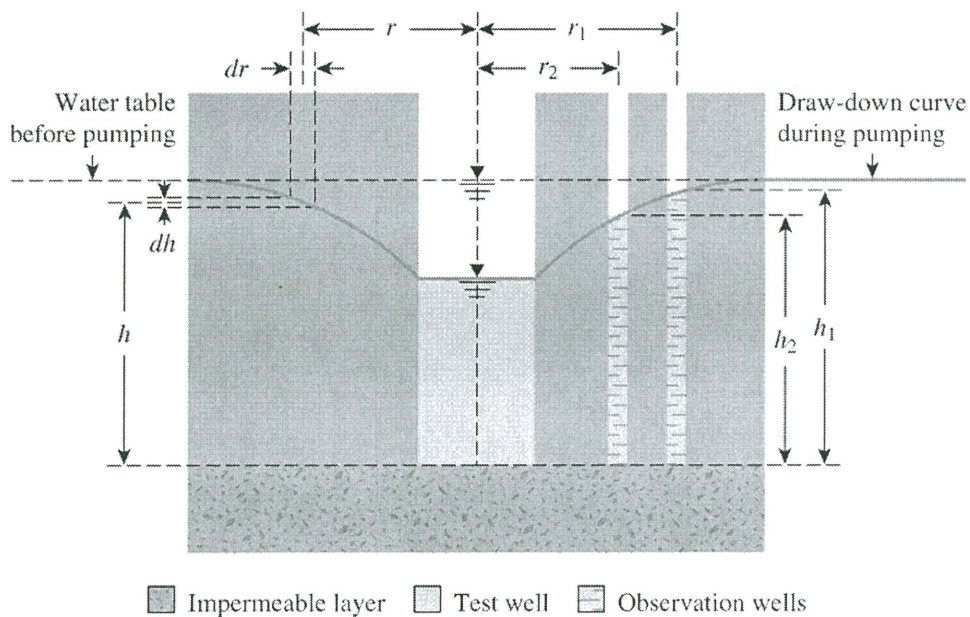
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List of formula:

$$k = \frac{QL}{Aht} ;$$

$$k = 2.303 \frac{aL}{At} \log_{10} \frac{h_1}{h_2}$$



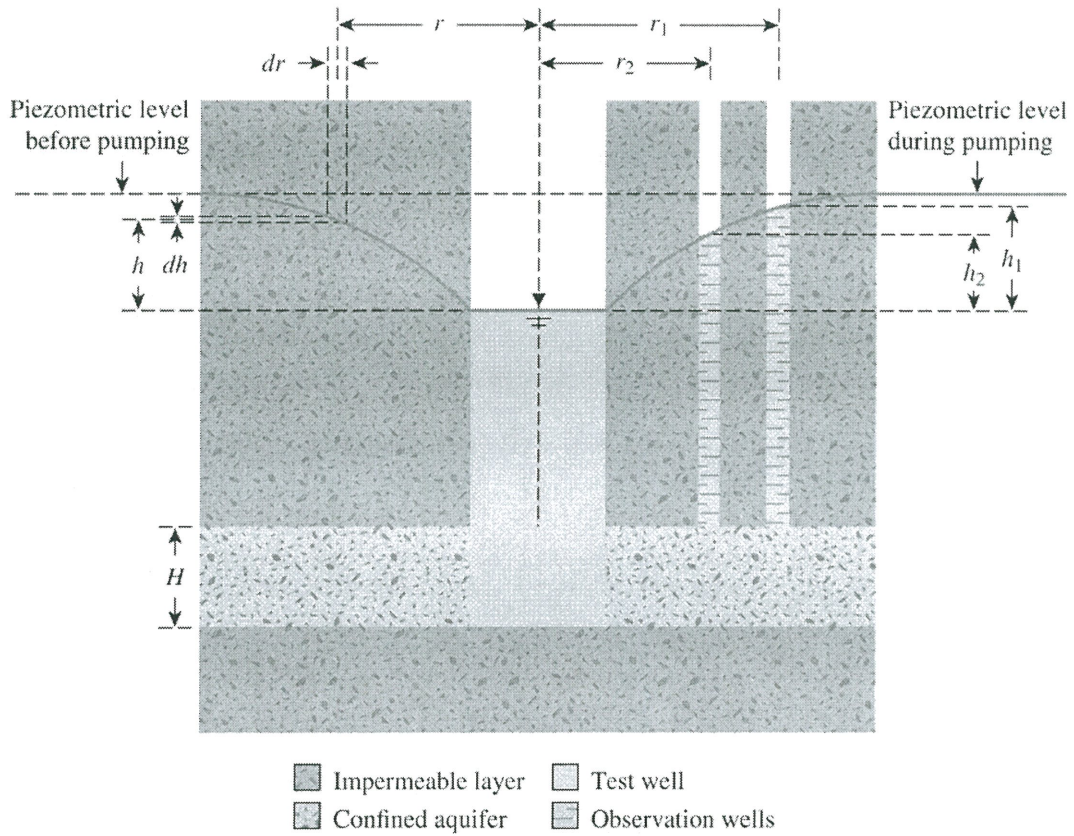
**FIGURE A:** Pumping test from a well in an unconfined permeable layer underlain by an impermeable stratum

$$k = \frac{2.303q \log_{10} \left( \frac{r_1}{r_2} \right)}{\pi(h_1^2 - h_2^2)}$$

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**FIGURE B:** Pumping test from a well penetrating the full depth in a confined aquifer

$$k = \frac{q \log_{10} \left( \frac{r_1}{r_2} \right)}{2.727 H (h_1 - h_2)}$$



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$$Cu = \frac{D_{60}}{D_{10}}; Cc = \frac{D_{20}^2}{D_{10}D_{60}}$$

$$R = \frac{\gamma_{d(field)}}{\gamma_{d(max-lab)}}; Dr = \left[ \frac{\gamma_{d(field)} - \gamma_{d(min)}}{\gamma_{d(max)} - \gamma_{d(min)}} \right] \left[ \frac{\gamma_{d(max)}}{\gamma_{d(field)}} \right]$$

Dry density for plotting Zero air void,

$$\rho_d = \frac{1 - \frac{V_a}{100}}{\frac{1}{\rho_s} + \frac{W}{100\rho_w}}$$

where

- $\rho_d$  is the dry density (in Mg/m<sup>3</sup>);
- $\rho_s$  is the particle density (in Mg/m<sup>3</sup>);
- $\rho_w$  is the density of water (in Mg/m<sup>3</sup>), assumed equal to 1;
- $V_a$  is the volume of air voids in the soil, expressed as a percentage of the total volume of the soil (equal to 0 %, 5 %, 10 % for the purpose of this plot);
- $w$  is the moisture content (in %).

Various forms of relationship for  $\gamma$ ,  $\gamma_d$  and  $\gamma_{sat}$

Moist unit weight ( $\gamma$ )		Dry unit weight ( $\gamma_d$ )		Saturated unit weight ( $\gamma_{sat}$ )	
Given	Relationship	Given	Relationship	Given	Relationship
$w, G_s, e$	$\frac{(1+w)G_s\gamma_w}{1+e}$	$\gamma, w$	$\frac{\gamma}{1+w}$	$G_s, e$	$\frac{(G_s+e)\gamma_w}{1+e}$
$S, G_s, e$	$\frac{(G_s+Se)\gamma_w}{1+e}$	$G_s, e$	$\frac{G_s\gamma_w}{1+e}$	$G_s, n$	$[(1-n)G_s+n]\gamma_w$
$w, G_s, S$	$\frac{(1+w)G_s\gamma_w}{1+\frac{wG_s}{S}}$	$G_s, n$	$G_s\gamma_w(1-n)$	$G_s, w_{sat}$	$\left(\frac{1+w_{sat}}{1+w_{sat}G_s}\right)G_s\gamma_w$
$w, G_s, n$	$G_s\gamma_w(1-n)(1+w)$	$G_s, w, S$	$\frac{G_s\gamma_w}{1+\left(\frac{wG_s}{S}\right)}$	$e, w_{sat}$	$\left(\frac{e}{w_{sat}}\right)\left(\frac{1+w_{sat}}{1+e}\right)\gamma_w$
$S, G_s, n$	$G_s\gamma_w(1-n)+nS\gamma_w$	$e, w, S$	$\frac{eS\gamma_w}{(1+e)w}$	$n, w_{sat}$	$n\left(\frac{1+w_{sat}}{w_{sat}}\right)\gamma_w$
		$\gamma_{sat}, e$	$\gamma_{sat} - \frac{e\gamma_w}{1+e}$	$\gamma_d, e$	$\gamma_d + \left(\frac{e}{1+e}\right)\gamma_w$
		$\gamma_{sat}, n$	$\gamma_{sat} - n\gamma_w$	$\gamma_d, n$	$\gamma_d + n\gamma_w$
		$\gamma_{sat}, G_s$	$\frac{(\gamma_{sat} - \gamma_w)G_s}{(G_s - 1)}$	$\gamma_d, G_s$	$\left(1 - \frac{1}{G_s}\right)\gamma_d + \gamma_w$
		$\gamma_{sat}, w_{sat}$		$\gamma_d, w_{sat}$	$\gamma_d(1+w_{sat})$