

**CONFIDENTIAL**



**UNIVERSITI TUN HUSSEIN ONN MALAYSIA**

**FINAL EXAMINATION  
SEMESTER I  
SESSION 2013/2014**

**COURSE NAME** : DESIGN OF WASTEWATER  
ENGINEERING

**COURSE CODE** : BFA 40403

**PROGRAMME** : 4 BFF

**EXAMINATION DATE** : DECEMBER 2013/JANUARY 2014

**DURATION** : 3 HOURS

**INSTRUCTION** : ANSWER **FOUR (4)** QUESTIONS  
ONLY

THIS PAPER CONSISTS OF **SIX (6)** PRINTED PAGES

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- Q1**
- (a) Explain the factors that will consider for selecting location of manhole. (5 marks)
- (b) State the significant physical, chemical and biological parameters to characterize wastewater. (6 marks)
- (c) Discuss the design criteria used to calculate the reactor volume and to evaluate the performance of biological processes. (8 marks)
- (d) A diameter of clarifier is 18.0 m and an average wastewater flow rate is 40.0 MLD. Calculate the hydraulic detention time and surface loading rate of a clarifier having a wastewater depth of 2.5 m in the clarifier. (6 marks)

- Q2**
- (a) Draw a flow diagram of a typical preliminary treatment system. (3 marks)
- (b) State **FOUR (4)** types of aerated grit chamber. (4 marks)
- (c) Explain advantages and disadvantages of the aerated grit chamber. (6 marks)
- (d) It is proposed to provide an aerated grit chamber for a treatment plant expecting an average of 10 MLD flow of domestic wastewater. Determine the dimensions of the grit chamber, the air requirement, and the quantity of grit to be removed.

*Given:*

- Peak flow factor = 2.5
- No. of chambers = 2 units
- Depth of chamber = 2 m
- Width-depth ratio = 1.5 : 1
- An air supply rate =  $0.3 \text{ m}^3/\text{min.m}$  (length of chamber)
- Grit settling =  $0.15\text{m}^3$  of grit per  $1000\text{m}^3$

(12 marks)

- Q3** (a) Explain the principal factors that must be considered in the design of equalization basins  
(5 marks)
- (b) A treatment plant being designed for a new town requires an equalization basin to even out flow variations. **Table 2** shows the typical flow of the average variation over a day.
- (i) Determine the equalization basin volume required for a uniform outflow equal to the average daily flow. Assume the flows are hourly averages and an addition of 25% to the estimated volume will be provided to account for contingencies.  
(10 marks)
- (ii) Design dimension of the equalization basin.  
(10 marks)

Table 2 : Hourly flow pattern

Time (hours)	Flow (m <sup>3</sup> /s)	Time (hours)	Flow (m <sup>3</sup> /s)
0000	0.0481	1200	0.0718
0100	0.0359	1300	0.0744
0200	0.0226	1400	0.0750
0300	0.0187	1500	0.0781
0400	0.0187	1600	0.0806
0500	0.0198	1700	0.0843
0600	0.0226	1800	0.0854
0700	0.0359	1900	0.0806
0800	0.0509	2000	0.0781
0900	0.0631	2100	0.0670
1000	0.0670	2200	0.0583
1100	0.0682	2300	0.0526

- Q4** (a) Discuss the significant factors that are normally considered in the design of aerobic biological process. (10 marks)
- (b) Explain the advantages of Sequencing Batch Reactor (SBR) system for municipal wastewater treatment. (5 marks)
- (c) An extended aeration system has been proposed to treat a wastewater flow of  $500 \text{ m}^3/\text{d}$ , with soluble BOD of  $150 \text{ mg/l}$ . Determine the concentration of soluble substrate and the volume of aeration tank.  
(Given:  $\text{MLSS} = 4000 \text{ mg/l}$ ,  $Y_t = 0.4$ ,  $K_d = 0.03 \text{ d}^{-1}$ ,  $K = 0.1 \text{ mg/l}$ ) (10 marks)
- Q5** (a) Explain the following processes for sludge treatment
- (i) Preliminary operations
  - (ii) Thickening
  - (iii) Stabilization
  - (iv) Conditioning
  - (v) Dewatering
- (10 marks)
- (b) Explain the following processes in wastewater treatment plant
- (i) Aerobic digestion
  - (ii) Anaerobic digestion.
- (6 marks)
- (c) Determine the daily volume of methane and total gas produced in an anaerobic digester that is operated at  $35^\circ \text{C}$  under the following conditions:
- Biosolids flow =  $300 \text{ m}^3/\text{d}$ ;  
 COD =  $5,000 \text{ g/m}^3$ .  
 $Y_{obs} = 0.04 \text{ g VSS/g COD}$   
 COD removal = 95%  
 Volume of methane gas = 65% of the total gas volume  
 COD of waste activated sludge = 1.42 of waste activated sludge produced ( $P_x$ )  
 $P_x = Y_{obs} Q (S_o - S)$
- (9 marks)

- END OF QUESTION -

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**SELECTION TABLE FOR FLOATING MECHANICAL AERATORS**

Size, kW	OTR <sup>b</sup> , kg/MJ	Nominal operating, depth, m	Complete mix zone, m	Complete O <sub>2</sub> dispersion zone, m
0.75	0.20	1.8	6	20
1.5	0.23	1.8	8	30
2.5	0.23	1.8	12	45
3.5	0.23	1.8	14	50
5.5	0.22	2.4	15	50
7.5	0.20	3.0	15	55
10	0.21	3.0	19	60
15	0.19	3.0	22	70
20	0.20	3.0	24	80
25	0.21	3.0	26	85

<sup>a</sup>These aerators are representative but do not represent actual choices. Actual manufacturers' data must be used for real world design.

<sup>b</sup>OTR = oxygen transfer rate

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**FLOATING AERATOR DEPTH REQUIREMENTS**

