



UTHM

Universiti Tun Hussein Onn Malaysia

UNIVERSITI TUN HUSSEIN ONN MALAYSIA

FINAL EXAMINATION SEMESTER I SESSION 2023/2024

COURSE NAME : WATER SYSTEM DESIGN AND
MANAGEMENT

COURSE CODE : BFC 35303

PROGRAMME CODE : BFF

EXAMINATION DATE : JANUARY/FEBRUARY 2024

DURATION : 3 HOURS

INSTRUCTION

1. ANSWER ALL QUESTIONS
2. THIS FINAL EXAMINATION IS CONDUCTED VIA
 Open book
 Closed book
3. STUDENTS ARE **PROHIBITED** TO CONSULT THEIR OWN MATERIAL OR ANY EXTERNAL RESOURCES DURING THE EXAMINATION CONDUCTED VIA CLOSED BOOK

THIS QUESTION PAPER CONSISTS OF **EIGHT (8)** PAGES

TERBUKA

CONFIDENTIAL

- Q1** (a) With the aid of sketches, explain **TWO (2)** differences between separate sewerage system, and combine sewerage system. (6 marks)
- (b) A sewerage reticulation plan for Taman Parit Raja as shown in **Figure Q1.1**.

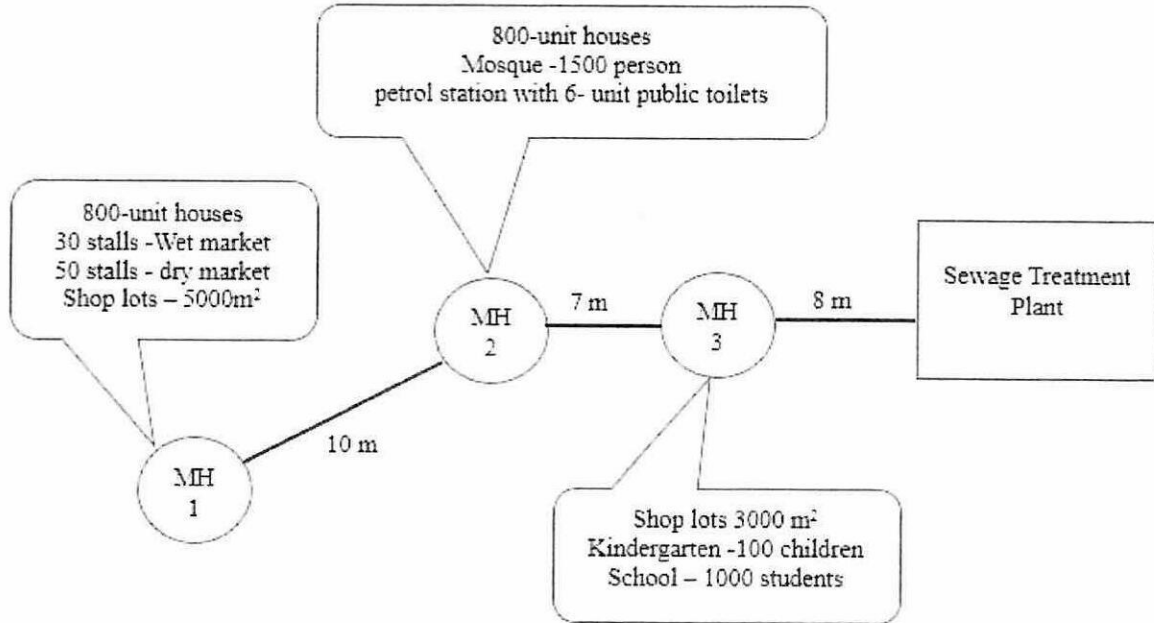


Figure Q1.1 This figure shows the sewerage reticulation plan.

- (i) Based on **Figure Q1.1** and **Table Q1.1**, calculate total population equivalent (PE) for the proposed sewerage reticulation plan.

Table Q1.1 This table shows the population equivalent.

| Type of establishment | Population equivalent |
|---|---|
| Residential | 5 per house |
| Commercial: Includes offices, shopping complex, entertainment/ recreational centres, restaurants, cafeteria and theatres | 3 per 100 m ² gross area |
| School/ Educational institutions: - Day schools/ institutions - Fully residential - Partial residential | 0.2 per student 1 per student 0.2 per non-residential student |
| Market (wet type) | 3 per stall |
| Market (dry type) | 1 per stall |
| Petrol kiosks/ service stations | 15 per toilet |
| Mosque/ church/ temple | 0.2 per person |

(8 marks)

- (ii) Based on **Figure Q1.1**, recommend a sewerage reticulation system for MH1 to MH2 using new vitrified clay sewer ($n = 0.010$) running at full flow condition. Assume ground level and upper invert level are 25.200 m and 24.055 m, respectively.

(11 marks)

- Q2** (a) Based on your understanding, explain the stormwater management mechanism and implementation in minimizing the effects of land use changes.

(6 marks)

- (b) **Figure Q2.1** shows drainage catchment of Wangsa Maju. Calculate time of concentration, t_c at Subcatchment 1. The slope of overland surface is 3.74% and the friction slope of the channel is 0.02. From the site visit, the depth and the width of the triangular channel are 0.3 m and 0.6 m, respectively. Assume Horton's Roughness, n and manning coefficient, n are both equal to 0.015. The proposed channel geometry for the drainage system **Figure Q2.2**.

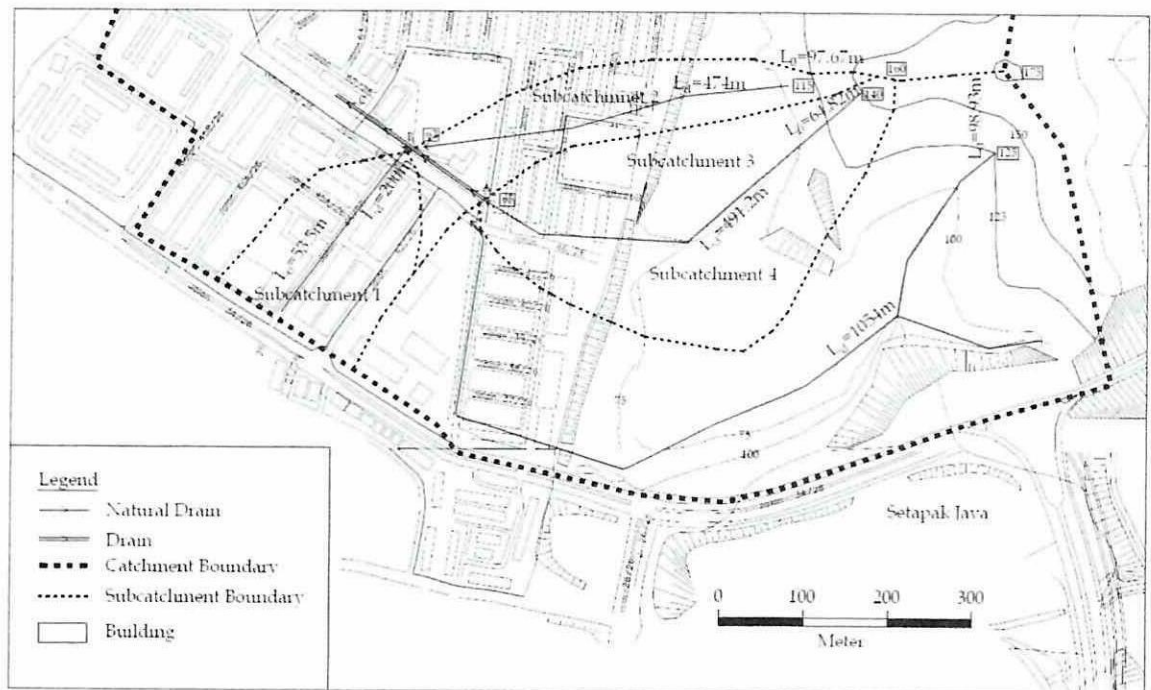


Figure Q2.1 This figure shows the sub catchment drainage.

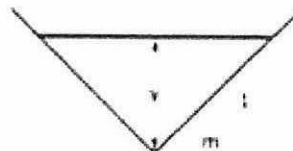


Figure Q2.2 This figure shows the dimensions of channel geometry.

(8 marks)

(c) A lined drain is proposed for a new residential project in Lendu, Melaka to convey design flow for 19 year ARI minor system. Assume $B = 0.7$ m, recommend the size of lined drain that could cater the discharge after the development. Use the following:

- 1) Post-development time of concentration, t_c = 45 minutes
- 2) Runoff coefficient, C = 0.8
- 3) Manning coefficient, n = 0.015
- 4) Side slope, Z = 0
- 5) Minimum freeboard = 50 mm
- 6) λ = 72.163
- 7) κ = 0.184
- 8) θ = 0.376
- 9) η = 0.900

(11 marks)

Q3 (a) Erosion and Sediment Control Plan (ESCP) consisted of EIGHT (8) components as illustrated in **Figure Q3.1** Explain briefly any **THREE (3)**.

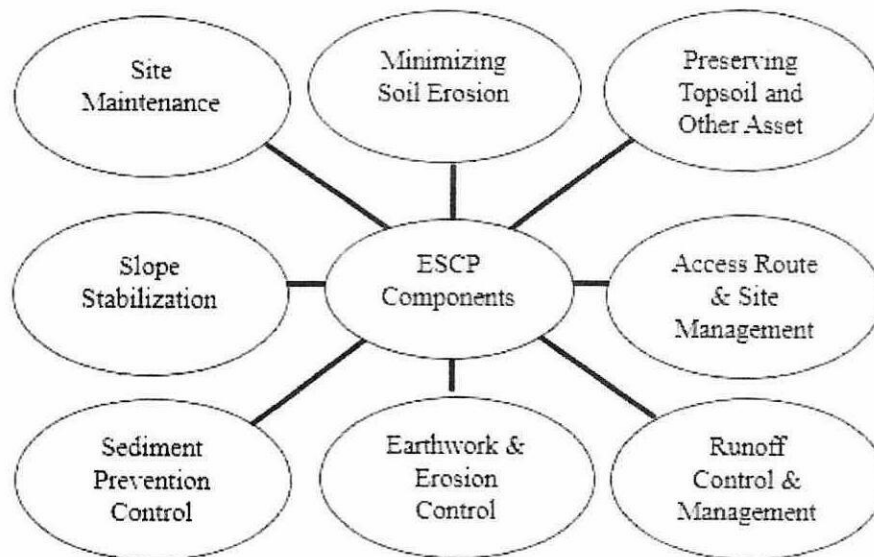


Figure Q3.1 This figure shows the ESCP components.

(6 marks)

(b) (i) The sediment basin of a construction area was built at 90% efficiency to manage the sediment yield of 80 tonne. Consider the density of sediment and sediment storage zone of 1600kg/m^3 and 900 m^3 respectively. Estimate the sediment trapping efficiency.

(4 marks)

- (ii) Provide an illustration of a control facility for erosion and sediment control within a construction area which is having earthwork activity. In the illustration, propose the best management practice (BMP) for erosion and sediment and place the control facilities.

(4 marks)

- (c) (i) Recommend settling zone of dry sediment basin for a construction area of 5 ha by considering design criteria of **Table Q3.1** and **Table Q3.2**. Assume time of concentration of basin catchment is 30 minutes.

Table Q3.1 This table shows Sediment basin design criteria.

| Parameter | Requirement |
|-----------------|---|
| Storage volume | Settling zone volume = half of total storage Sediment zone volume = half of total storage |
| Basin dimension | Minimum length to width ratio= 2:1 Minimum length to settling depth ratio= 200:1 Minimum settling zone depth =0.6 m Minimum sediment storage zone depth =0.3 m |
| Embankment | Side slope 2 (H): 1 (V) or flatter |

Table Q3.2 This table shows the time of concentration of basin catchment.

| Parameter | Time of Concentration of Basin Catchment (minutes) | | | | |
|-----------------------------------|--|-----|-----|-----|-----|
| | 10 | 20 | 30 | 45 | 60 |
| Surface area (m ² /ha) | 333 | 250 | 200 | 158 | 121 |
| Total volume (m ² /ha) | 400 | 300 | 240 | 190 | 145 |

(9 marks)

- (ii) Based on the design in Q3(c)(i), comment on whether the sediment basin dimension remain the same if the time of concentration is changed to 45 minutes

(2 marks)

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- Q4** (a) Grid system (or looped system) is one of the water distribution systems. Explain **THREE (3)** benefits of this system. (6 marks)
- (b) A new development project consisting of residential and commercial premises is proposed. Using **Table Q4.1**, calculate the water demand for the project with establishment as in **Table Q4.2**, and estimate the peak flow.

Table Q4.1 This table shows average daily water demand.

| Type of Premises/Buildings | Average Daily Water Demand (Litres) |
|--|-------------------------------------|
| Single storey terrace house/low cost house (less than RM 25,000)/ low medium & medium cost flats | 1300/unit |
| Double storey terrace house/high cost flat/apartment/town house | 1500/unit |
| Semi detached house/cluster | 2000/unit |
| Wet market | 1500/stall |
| Dry market | 450/stall |
| Shop house (single storey) /low-cost shop | 2000 /unit |
| Shop house (double storey) | 3000/unit |
| Shop house (three storey) | 4100/unit |
| Office/complex/commercial (domestic usage) | 1000/100 square meter |
| Community centres or halls | 1000/100 square meter |
| Petrol kiosk (with car washing bay) | 50000/unit |
| Petrol kiosk (without car washing bay) | 10000/unit |

Table Q4.2 This table shows the quantity for different premises.

| Type of Premises/Buildings | Unit |
|--|---------------------|
| Single storey terrace house | 1000 |
| Double storey terrace house | 500 |
| Semi detached house | 200 |
| Wet market | 40 |
| Dry market | 100 |
| Shop house (single storey) | 100 |
| Shop house (double storey) | 100 |
| Office/complex/commercial (domestic usage) | 2000 m ² |
| Community centres or halls | 500 m ² |
| Petrol kiosk (without car washing bay) | 1 |

(8 marks)

(c) (i) Recommend the available pressure at location of pressure test and available pressure at location of proposed connection point for a purposed development project using the following:

- 1) Residual Pressure : 28.0 m
- 2) Ground level : 25.20 m
- 3) Depth of pipe : 1.0 m
- 4) Pipe length : 3 km
- 5) Head loss in pipe due to friction : 2m/1000m

(4 marks)

(ii) For the closed loop as shown in **Figure Q4.1** with $K = 500$, find discharge in all pipes. Limit to **TWO (2)** iterations.

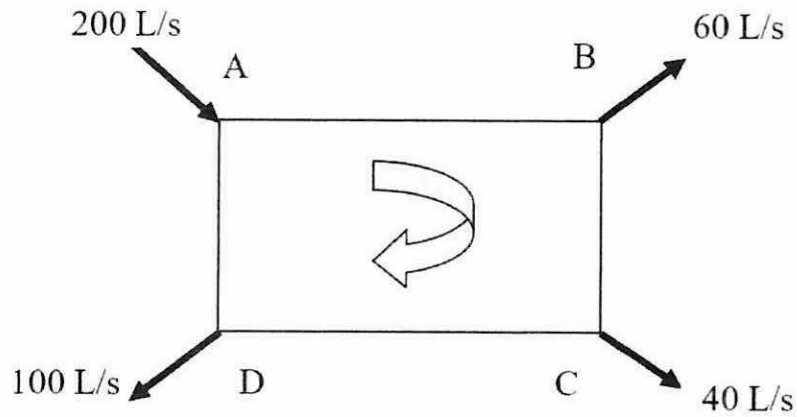


Figure Q4.1 This figure shows the closed loop for the water distribution.

(7 marks)

-END OF QUESTIONS-

